## A development of two-way coupling model between wave overtopping-induced surface flow and backflow of sewer systems in coastal cities

### Junbeom Jo

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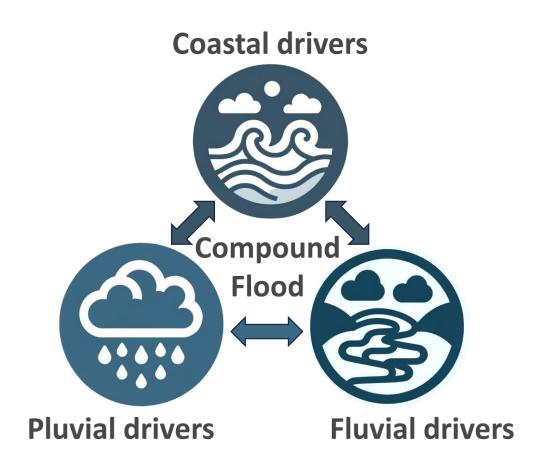
Coastal Disaster Risk Research Laboratory

Disaster Prevention Research Institute





## Why Do We Need Fully Coupled Models for Flood Simulation?



Coastal Urban Flooding is driven by complex interaction.

### **X** Conventional (One-way) Models:

 Ignore feedback effects among flood drivers and oversimplify real dynamics

### **✓** Fully Coupled (Two-way) Models:

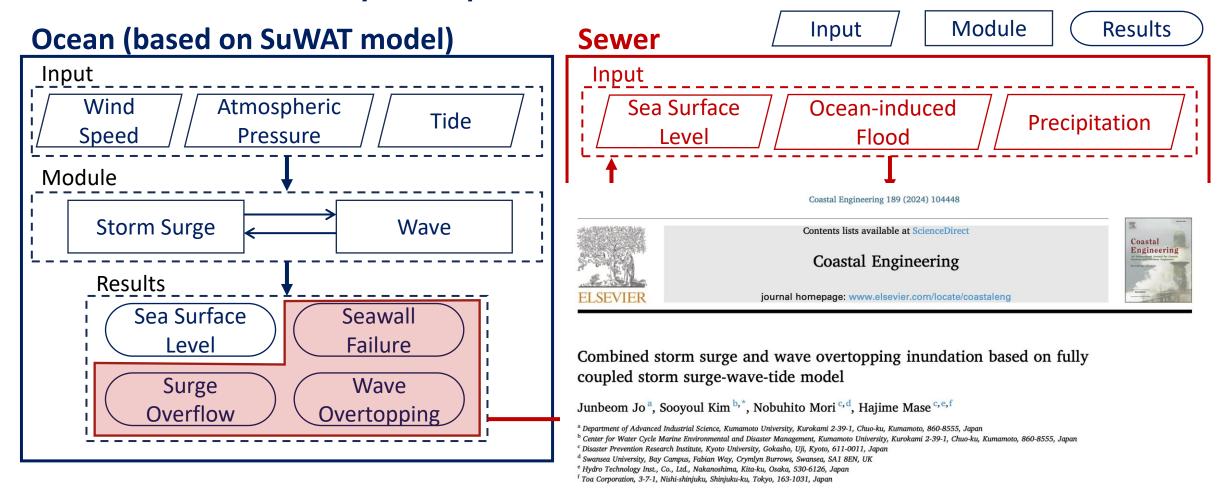
 Capture bidirectional interactions between Coastal-Pluvial-Fluvial drivers

### What Happens Without Coupling?

 Can lead to both over- and underestimation of compound flood risk

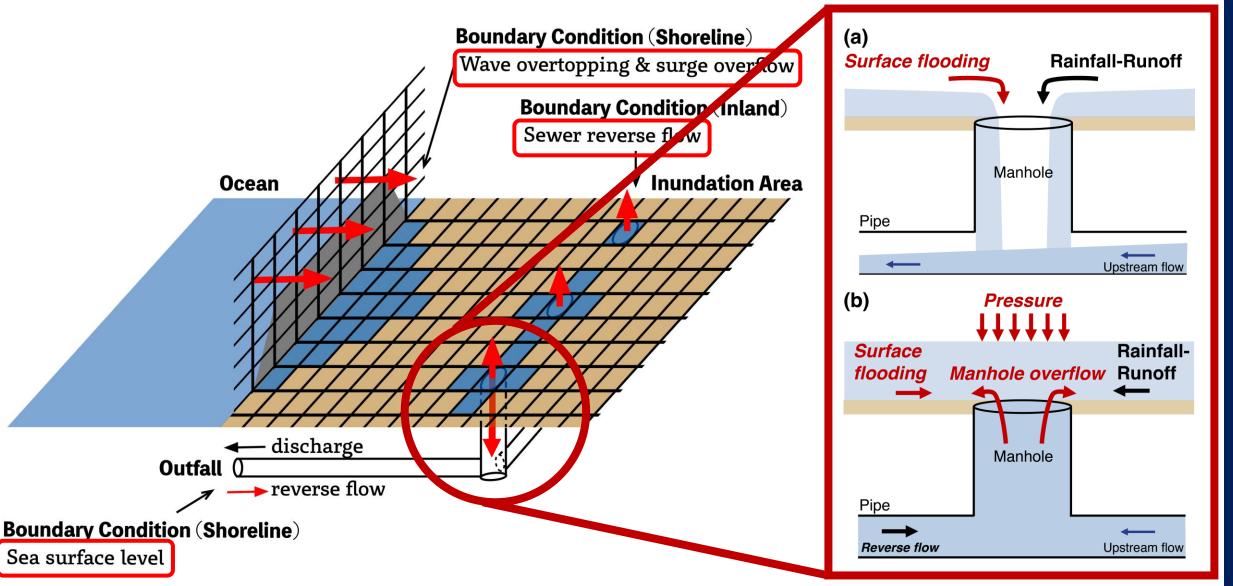
One-way models cannot account for real flood dynamics.

## Flowchart for Fully Coupled Flood Model

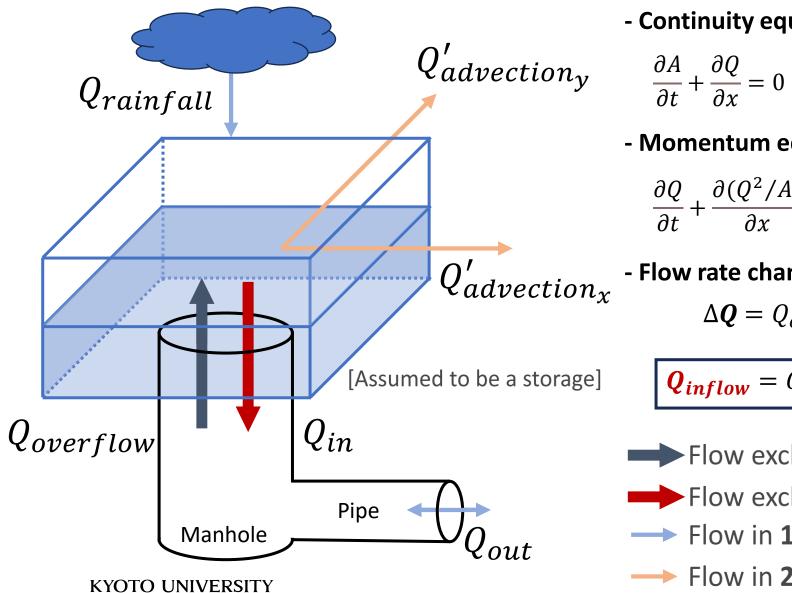


- The fully coupled model was developed based on the Ocean-induced flood model.
- Sewer flow module was integrated for dynamic data exchange at each time step.

## Framework of Fully Coupled Flood Model



## Schematic Diagram for Flow Exchange



- Continuity equation (1D)

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = 0$$

- Momentum equation (1D)

t: time

A: cross-sectional area

Q : flow rate

H: hydraulic head

 $S_f$ : friction slope

 $h_L$ : local energy loss

$$\frac{\partial Q}{\partial t} + \frac{\partial (Q^2/A)}{\partial x} + gA\frac{\partial H}{\partial x} + gAS_f + gAh_L = 0$$

- Flow rate change in the sewer system (1D-2D)

$$\Delta \mathbf{Q} = Q_{overflow} + \mathbf{Q}_{inflow} + Q_{pipe}$$

$$Q_{inflow} = Q_{rainfall} + Q'_{advection_x} + Q'_{advection_y}$$

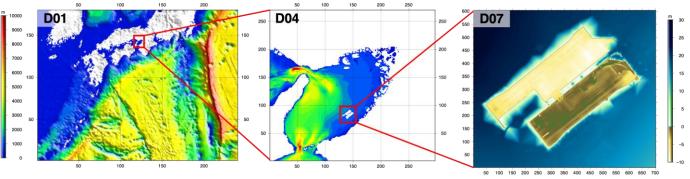
- Flow exchange from 1D to 2D
- Flow exchange from 2D to 1D
- → Flow in **1D**
- → Flow in **2D**

## Study Area and Bathymetry: Kansai Airport, Japan





- Wave overtopping of seawalls
- Localized seawall failure
- Sewer-induced reverse flow of seawater and rainwater



< Bathymetry for Grid refinement >

Domain	Grid Size	Refinement Ratio	
D01	7200 m	-	
D02	2400 m	1:3	
D03	810 m	≈ 1:3	
D04	270 m	1:3	ر ا
D05	90 m	1:3	
D06	30 m	1:3	
D07	10 m	1:3 -	L

## Time step determination (CFL condition)

$$CFL = u_x \frac{\Delta t}{\Delta x} \le C_{max}$$

$$C_{max} = 1$$

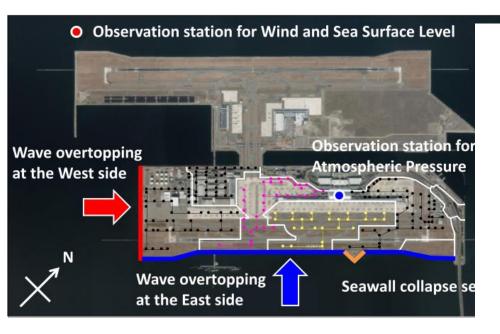
 $u_x$ : the velocity (=  $\sqrt{2gh_x}$ 

 $\Delta t$ : the time step

 $\Delta x$ : the grid size

0.4 s was determined.

## Study Area and Bathymetry: Kansai Airport, Japan



### Flood drivers (by TC Jebi(2018)):

- Wave overtopping of seawalls
- Localized seawall failure
- Sewer-induced reverse flow of seawater and rainwater

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Research papers

Coastal and pluvial compound flooding of surge, wave, tide, rainfall-runoff, sewer flow, and seawall failure at coastal urban areas

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#### ARTICLE INFO

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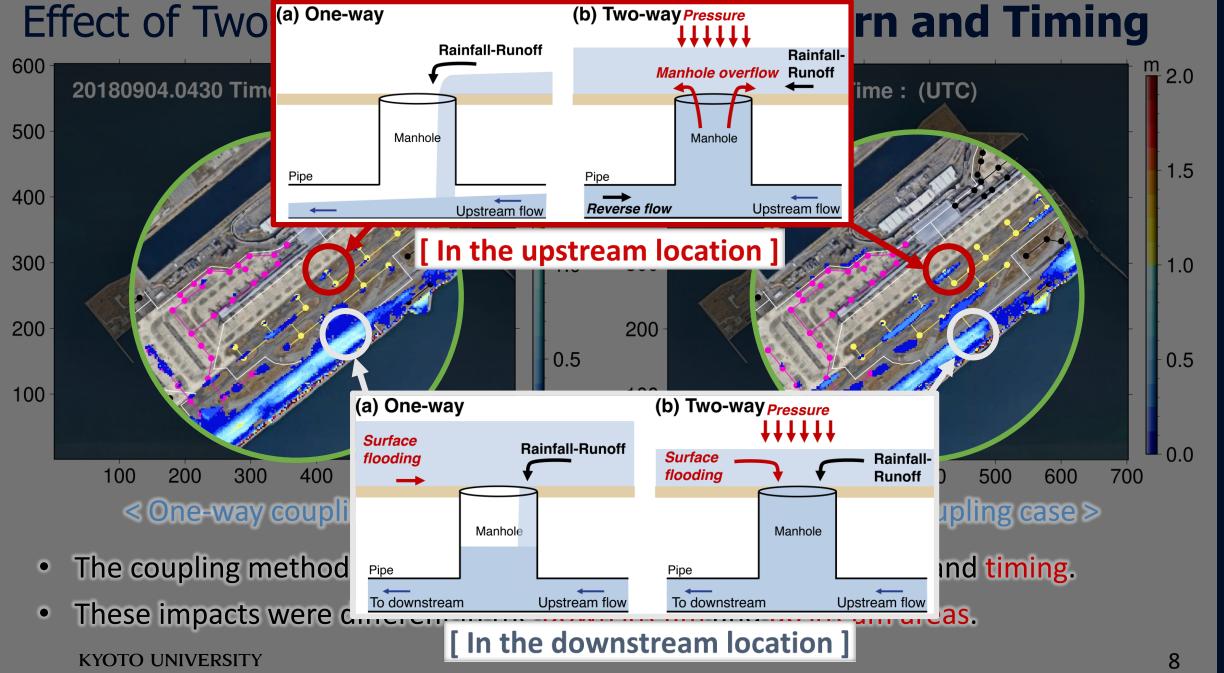
Surge-wave-tide interaction Sewer system reverse flow Surface rainfall-runoff Multi-hazard assessment Compound flooding

#### ABSTRACT

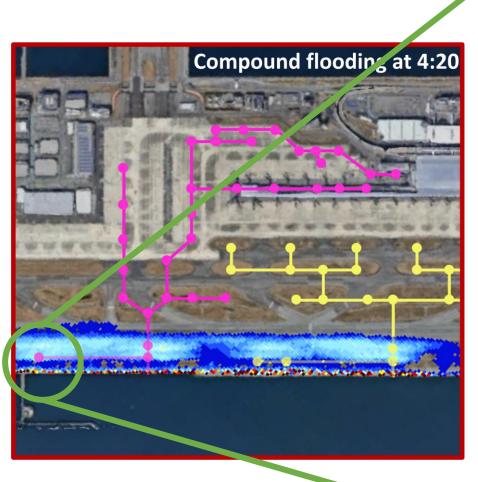
This study examines the compound storm surge, storm surge and sewage flooding at Kansai International Airport during Typhoon Jebi in 2018 using the SuWAT model, which integrates storm surge, tides, waves, wave overtopping, and pluvial processes. The findings for Typhoon Jebi case indicate that wave overtopping was the primary contributor to the flooding, accounting for 90.8% of the total flood volume, followed by seawall failure (7.5%) and sewer reverse flows (1.7% and 0.26% from seawater and rainwater, respectively). The multi-process coupled model demonstrated greater accuracy in flood depth estimations compared to a simple summation of individual components, underscoring the significance of accounting for interactions among flood drivers. The study highlights the need for integrating multiple flood mechanisms to improve hazard assessment in coastal urban areas. It suggests future enhancements, such as two-way coupling between surface inundation and sewer dynamics, to understand complex flooding processes further and develop better disaster hazard reduction strategies.

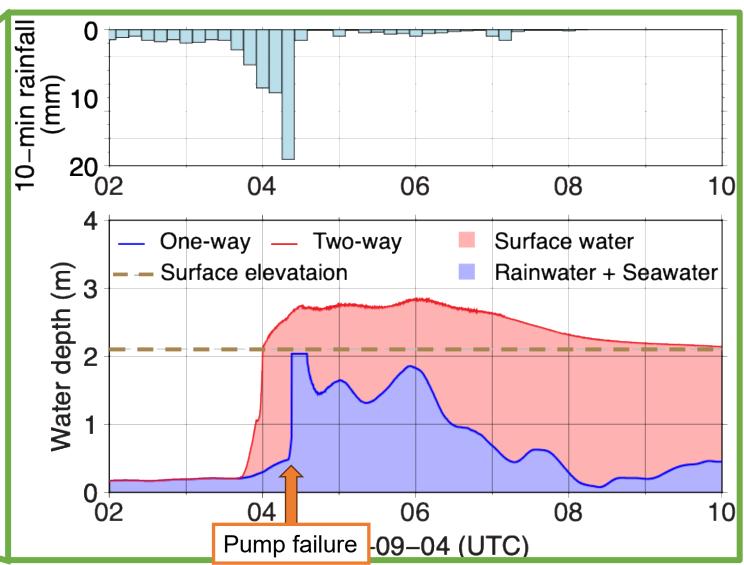
Reproducing Jebi-induced compound flooding using one-way coupling model



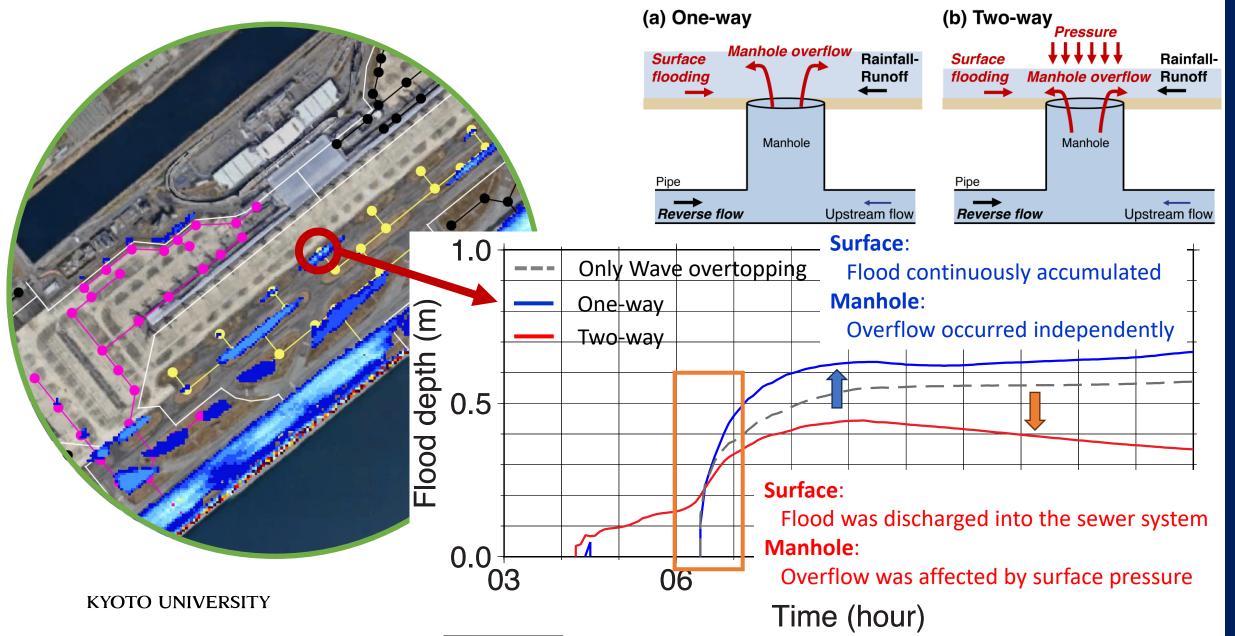


## Effect of Two-way Coupling on Manhole Water Depth

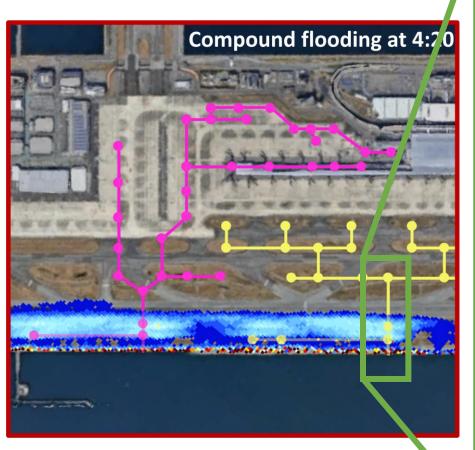


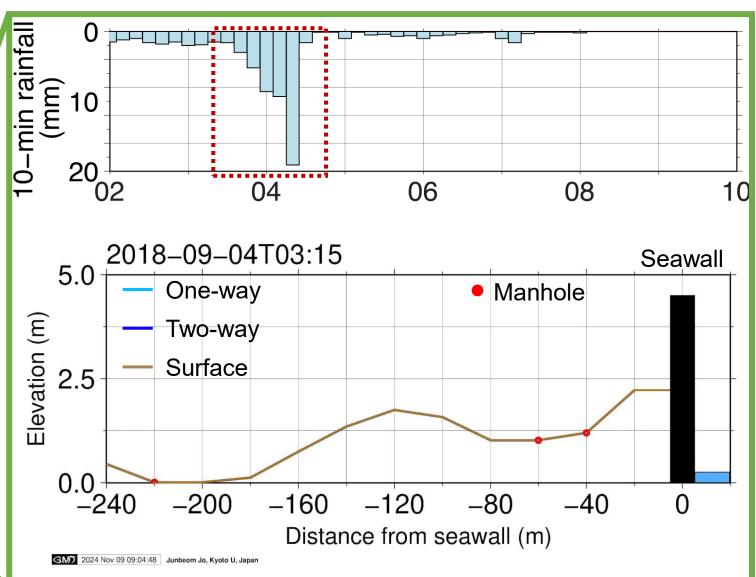


## Effect of Two-way Coupling on Surface Flood Depth



## Flood Depth Variation Along Profile Line: Start of Flooding

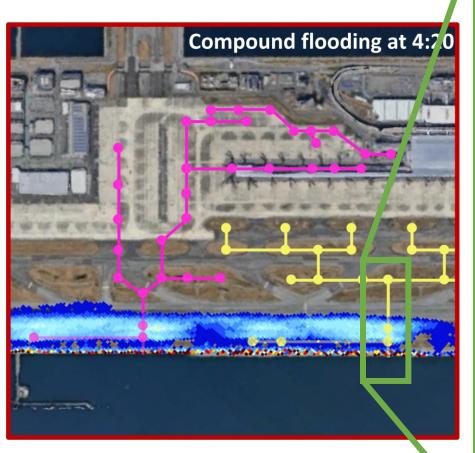


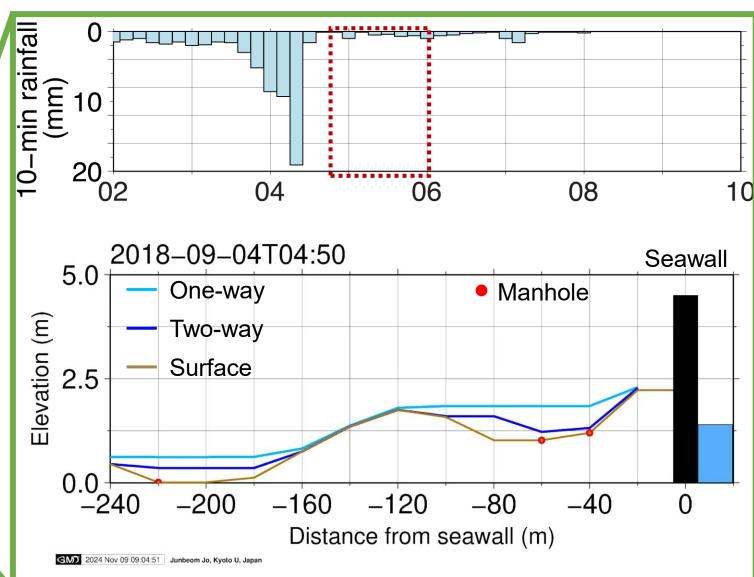


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## Flood Depth Variation Along Profile Line: Peak of Flooding

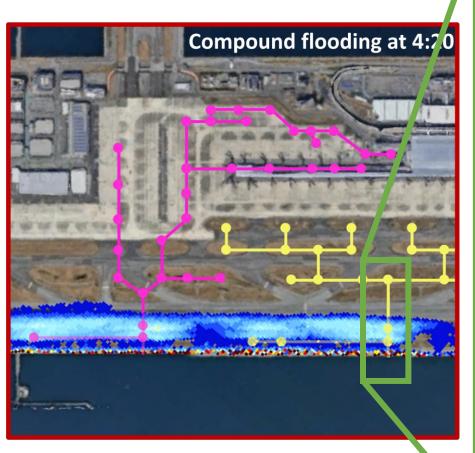


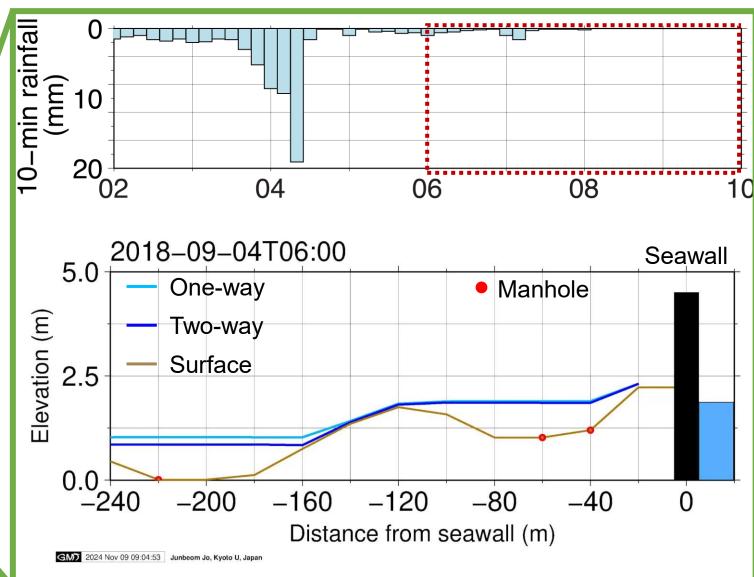


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## Flood Depth Variation Along Profile Line: End of Flooding





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Two-way

One-way

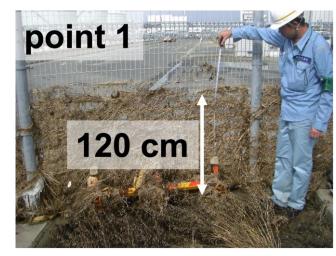
## **Peak Flood Depth** Compared with Field Survey

2.0

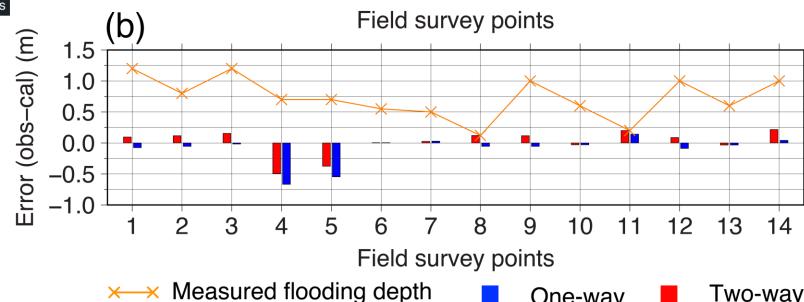


Peak flood depth (m) 1.5 1.0 0.5 0.0 10

< Location of field survey points >

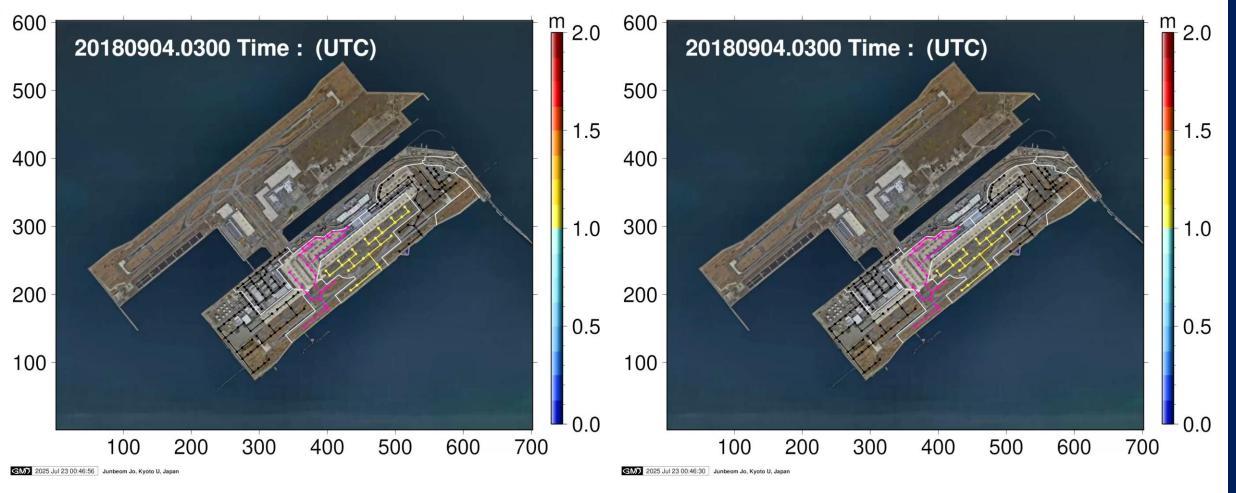


< Field survey overview >



## One-Way vs Two-Way Flood Simulation

- Simulations based on identical forcing conditions – only the coupling differs.



< One-way coupling case >

< Two-way coupling case >

## Conclusion

- The fully coupled flood model incorporating surface, ocean, and sewer systems was developed based on SuWAT model.
- The implementation of two-way coupling revealed complex flood dynamics, including:
  - ✓ Changes in the timing of flood
  - ✓ Redistribution of flood extent
  - ✓ Sewer reverse flow mechanisms
  - ✓ Drainage differences after the flood
- These results demonstrate the potential applicability of the proposed model for assessment of compound flood risk in coastal urban area.

# Thank you for your attention

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