# **Force acting on the Vertical Structure during Overflow**



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# **1. Introduction**

#### Research necessity

- Overflow scenarios introduce complex challenges related to jet trajectories and air entrainment, demanding meticulous study.
- Existing analyses frequently overlook essential factors, especially sub-atmospheric pressure in the cavity, which can jeopardize structural stability.
- Moreover, current research reveals inaccuracies in calculating downstream water forces due to assumption of hydrostatic pressure, neglecting the influence of significant air bubble movement in air-water flows

## Research objectives

- Investigate the intricate fluid dynamics of air-water flow and cavity pressure properties during vertical structure overflow.
- Utilize experiments to quantify the force acting on the model vertical structure, using different water pressure profiles: 1) Hydrostatic, 2) Bernoulli, 3) Discretized Navier-Stokes.
- Utilize the Buckingham Pi theorem to establish non-dimensional relationships among variables for convenient application of the research findings.

# 2. Key Concepts

**Background review:** air entrainment mechanism (with Aerated and Non-aerated cases)

- As shown in Figure 1, according to Chanson (1996), air bubble entrainment occurs along the cavity interface and at the end of the cavity through a re-entrant jet mechanism, leading to negative pressure in the air cavity.



Downstream: Cavity pressure + Navier Stokes

Pressure acting on the structure

Upstream: Hydrostatic Pressure

### Downstream flow regime

Subcritical Downstream

The pattern of air cavity pressure and air-water pressure may differ with changes in the downstream flow regime conditions.



[Figure 1] (Left) Air entrainment mechanism (Center) Aerated case, (Right) Non-aerated case Reference: Chanson, H. (1996). Air bubble entrainment in free-surface turbulent shear flows. Elsevier.

Upstream Hydrostatic pressure effect depth effect (Stabilizing)

[Figure 2] Pressure Profile distribution

h<sub>d,sub</sub>

Supercritical Downstream

[Figure 3] Downstream flow regime (Left: Subcritical, Right: Supercritical)

# **3. Research Methodology**

#### **Experimental setup: Flume and model weir**

- As shown in Figure 1, air bubble entrainment occurs along the cavity interface and at the end of the cavity through a re-entrant jet mechanism, leading to negative pressure in the air cavity.



► Mathematical expression for different pressure profiles

Applying hydrostatic conditions is incorrect due to air bubble movement. In this study, we calculate and integrate 1) Hydrostatic, 2) Bernoulli, and 3) Navierstokes pressure, respectively, to determine the forces acting on the model Find the pressure profile that best matches the measured force. For Hydrostatic condition: Assuming steady, uniform, and inviscid conditions

## Experimental variables

The number of total experimental cases: 192 cases with different independent variables.

Independent variables and dependent variables are detailed in Table 1.

Variables	Values	Unit	How to measure?				
Flow discharge	8 different cases	[lpm]	Flowmeter				
Model weir height	(0.3, 0.35, 0.4)	[m]	Constants				
End weir height	(0, 0.05, 0.10, 0.15)	[m]	Constants				
Cavity aeration	Aerated or Non-aerated	—	—				
Upstream water depth relative to weir crest	Dependent	[m]	Gauge Sensor				
Air-water density	Dependent	$[kg/m^3]$	<b>Conductivity Probe</b>				
Air cavity pressure and air-water pressure	Dependent	[ <i>Pa</i> ]	Pressure sensor				
Tailwater depth	Dependent	[m]	Image processing method				
Velocity close to the wall	Dependent	[m/s]	BIV method				
Force acting on the box	Dependent	[ <i>N</i> ]	Load Cell				
[Table 1] Independent variables (in black) and dependent variables (in blue)							

#### Tailwater depth calculation

Object Tracking was utilized for fluctuating tailwater depth calculations,

$$\left(\frac{\partial v}{\partial t} + u\frac{\partial v}{\partial x} + v\frac{\partial v}{\partial y}\right) = -\frac{1}{\rho_m}\frac{\partial p}{\partial y} + v_t\left(\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial y^2}\right) - g \quad \Rightarrow \quad p_h = -\rho_m gy$$

For Bernoulli equation: Assuming steady and inviscid conditions

$$\left(\frac{\partial v}{\partial t} + u\frac{\partial v}{\partial x} + v\frac{\partial v}{\partial y}\right) = -\frac{1}{\rho_m}\frac{\partial p}{\partial y} + v_t\left(\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial y^2}\right) - g \quad \rightarrow \quad \frac{p_b}{\rho_m g} + \frac{V^2}{2g} + y = C$$

- For Navier-Stokes equation:

$$\left(\frac{\partial v}{\partial t} + u\frac{\partial v}{\partial x} + v\frac{\partial v}{\partial y}\right) = -\frac{1}{\rho_m}\frac{\partial p_n}{\partial y} + v_t\left(\frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial y^2}\right) - g$$

(FDM with Forward-time and Forward or Backward space scheme is used)

- Velocity calculations employed the Bubble Image Velocimetry (BIV) method from the PIVlab MATLAB toolbox.
- High-speed videos (120 fps) were used to capture bubble motion. A QR code provides access to a video demonstration. (Top: Aerated, Bottom: Non-aerated)

Velocity calculation



[Figure 5] Snapshot of velocity vectors (Left: Aerated, Right: Non-aerated)

## combining color threshold, edge detection, and the mean-shift tracking algorithm.

A QR code provides access to a video demonstration. (Top: Aerated, Bottom: Non-aerated)







[Figure 6] Snapshot of Tailwater depth (Left: Aerated, Right: Non-aerated)

# 4. Research Results

### Data acquisition results

- All data, including cavity pressure, air-water pressure, discharge, water depth, velocity, and more, were collected over a 30second duration. To facilitate visualization, the sampling rate was standardized to 10 Hz.
- There are noticeable differences between an aerated case and a non-aerated case, particularly in terms of cavity pressure.









Hvdrostatio

Navier Stokes

700

600

500

[Aerated] Comparison of Downstream Pressure Models - Time: 0.00 s

tmospheric pressure

200

300

400

Pressure (Pa)

0.25

0.20

분 0.15

0.10

0.05



[Figure 7] Air-Water Pressure, Cavity Pressure, Discharge, and Upstream Water Depth (Left: Aerated, Right: Non-aerated)

#### Pressure profile and tailwater depth at downstream wall of the model weir

Bernoulli and Navier-Stokes pressures tend to align, but there are differences compared to hydrostatic pressure. This suggests that the velocity term significantly influences pressure in this sheet flow.

0.25

0.20

E 0.15

0.10

0.05

0.00

[Figure 9] Contrasting pressure profiles and tailwater depth at downstream wall (Left: Aerated, Right: Non-aerated)

You can access a real-time pressure data video demonstration through a QR code (Top: Aerated, Bottom: Non-aerated).



		Coefficients	value	Lower bound	Upper bound
SSE	0.000	<i>C</i> <sub>00</sub>	-0.06	-0.53	0.40
<b>R-squared</b>	0.927	<i>C</i> <sub>10</sub>	-0.24	-1.37	0.89
Adjusted R-squared	0.781	<i>C</i> <sub>01</sub>	0.15	-0.66	0.96
RMSE	0.005				

Coefficients with 95% confidence bounds

Average

[Figure 8] Dimensionless equations for air cavity pressure (Top: Subcritical downstream, Bottom: Supercritical downstream)

Goodness of fi

#### **Force acting on the model weir**

Real-time forces obtained from various pressure profiles (Red: Hydrostatic, Green: Bernoulli, Blue: Navier-Stokes).

Navier-Stokes forces (Blue line) consistently align with measured values (Black line), regardless of aeration or flow rate.



[Figure 10] Net force acting on the model weir (Left: low flowrate, Right: high flowrate)

## **5.** Conclusions

There are significant differences between the aerated and non-aerated cases in terms of factors such as air cavity pressure, tailwater depth, and the forces acting on the box.

And the state of t

400

500

In subcritical and supercritical downstream scenarios, there exist distinct relationships between air cavity pressure and independent variables.

200

300

Pressure (Pa)

tmospheric pressure

In overflow scenarios like this study, it has been verified through comparisons with measured data that to accurately determine the pressure acting on the downstream wall, the Navier-Stokes equation must be utilized.