

P.P. Shirshov Institute of Oceanology,  
Russian Academy of Sciences  
<http://www.ocean.ru>

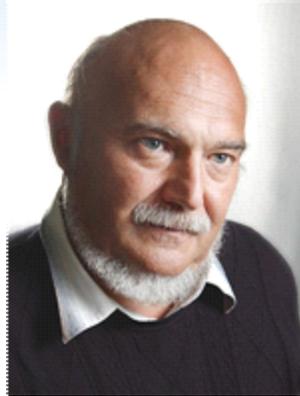


# **A PROJECT OF CONCRETE STABILIZED SPAR BUOY FOR MONITORING NEAR-SHORE ENVIRONMENT**





Sergei I. Badulin  
Wave theory



Andrey G. Zatsepin  
Experimental oceanography



Dmitry V. Ivonin  
Remote sensing



Dmitry G. Levchenko  
Seismo-acoustics



Alexander G. Ostrovskii  
Marine acoustics,  
technique



Vladislav V. Vershinin  
Civil engineering



Leopold I. Lobkovsky  
Geodynamics, geotectonics

# **A sea buoy of our dream -**

## **A stabilized research platform**

in the right place, at the right time  
(today, for the East near-shore of the Black Sea)

### **Durability (10 years min.)**

Mechanical strength

Corrosion

...

### **Sustainability**

Supportability

Multi-tasking

Upgradability

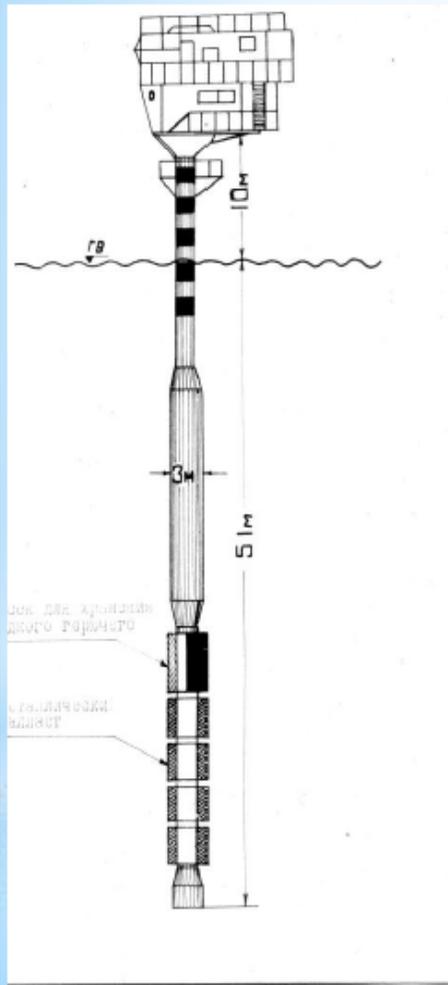
...

**Reasonable cost** of production, deployment and exploitation

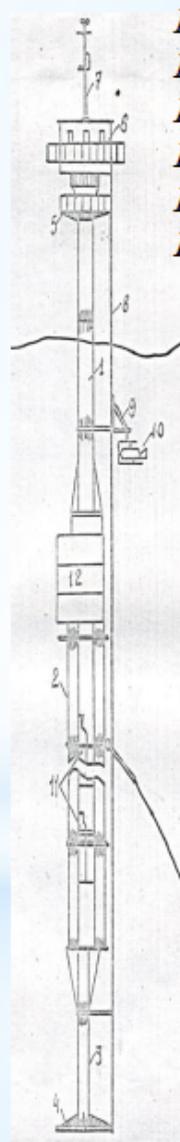
**Mass production (!?)**

# Stationary research buoys

France, 1963,  
H=2600 m

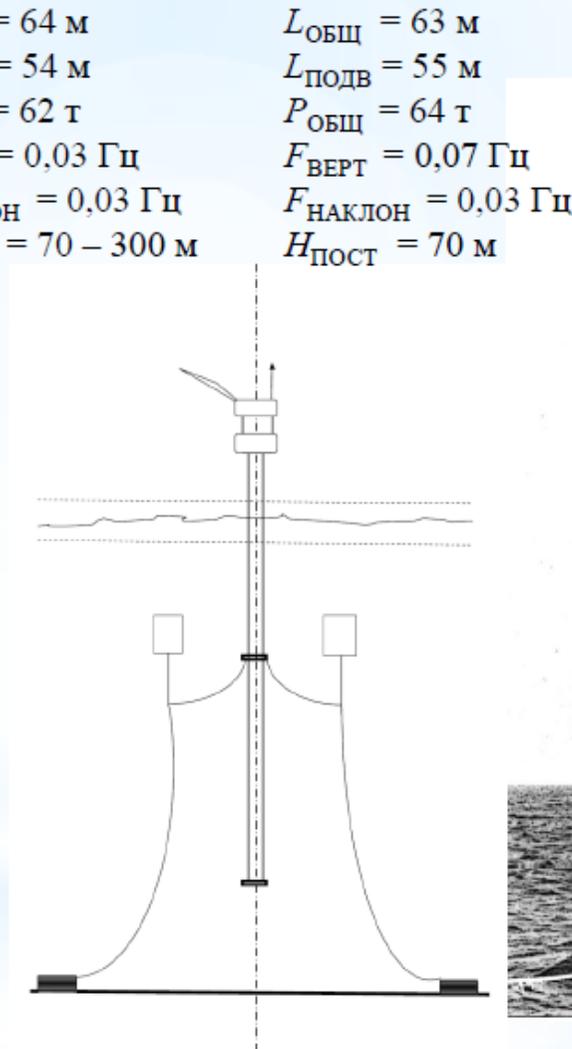


USSR, Institute of  
acoustics, 1977



$L_{\text{ОБЩ}} = 64 \text{ м}$   
 $L_{\text{ПОДВ}} = 54 \text{ м}$   
 $P_{\text{ОБЩ}} = 62 \text{ т}$   
 $F_{\text{ВЕРТ}} = 0,03 \text{ Гц}$   
 $F_{\text{НАКЛОН}} = 0,03 \text{ Гц}$   
 $H_{\text{ПОСТ}} = 70 - 300 \text{ м}$

USSR, P.P.Shirshov Institute,  
1974-1982

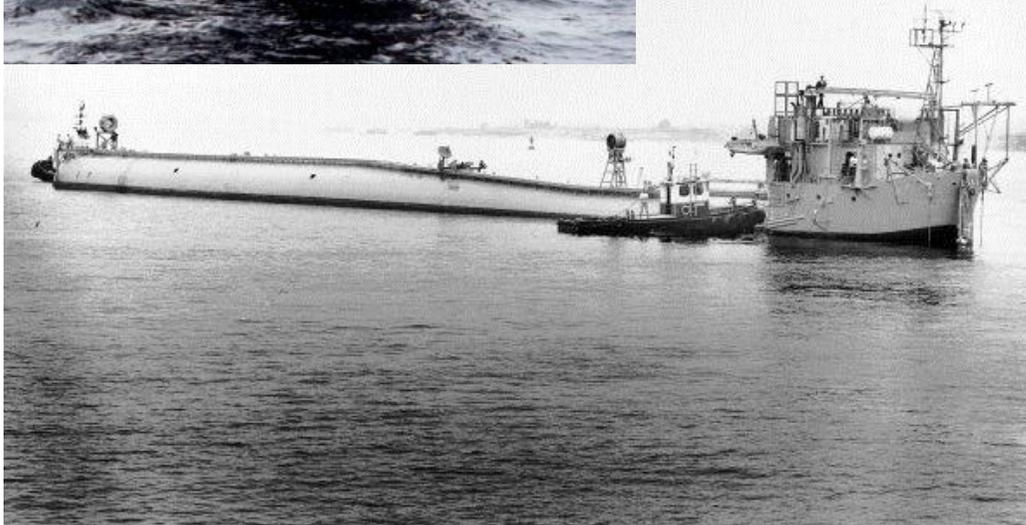


$L_{\text{ОБЩ}} = 63 \text{ м}$   
 $L_{\text{ПОДВ}} = 55 \text{ м}$   
 $P_{\text{ОБЩ}} = 64 \text{ т}$   
 $F_{\text{ВЕРТ}} = 0,07 \text{ Гц}$   
 $F_{\text{НАКЛОН}} = 0,03 \text{ Гц}$   
 $H_{\text{ПОСТ}} = 70 \text{ м}$

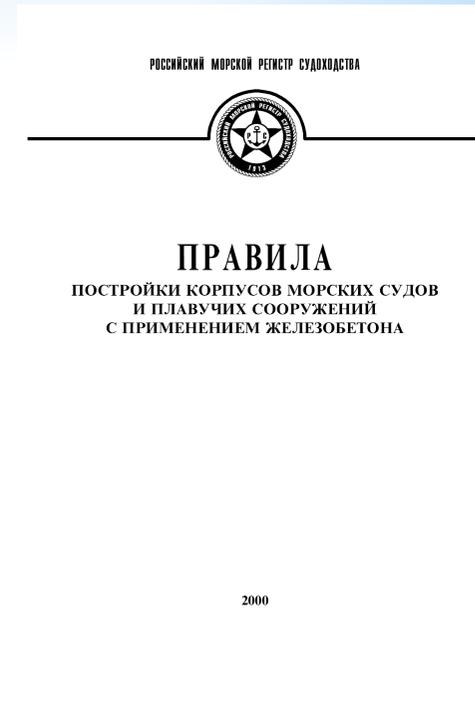


## Stabilized research platform FLIP USA, 1962-today

$L_{\text{tot}}=105\text{m}$   
 $L_{\text{sub}}=90\text{m}$   
 $P_{\text{tot}}=2200\text{ tonnes}$   
 $\Delta\alpha=2^\circ$   
 $\Delta h=0.1 H_s$   
 $W=2*100\text{ kWt}$   
Crew - 12 men



# Concrete (ferrocement) vessels (**decision**)



# Static of the buoy: buoyancy to weight aspect

## Parameters:

Concrete specific weight:  $\rho = 2.4 \text{ t/m}^3$   
 Radii (max):  $R = 1.2 \text{ m}$   
 Min. shell thickness:  $\Delta = 0.12 \text{ m}$   
 Section height:  $H_A = 3 \text{ m}$   
 Underwater part depth:  $H_B = 30 \text{ m}$

## Estimates of buoyancy:

$P_B \leq Q_B$  ( $P_B$  и  $Q_B$  – weight and buoyancy)

Buoyancy condition:  $k = \Delta/R < k_{lim} = 0.236$

### Ex. 1 (max. diameter $\Delta=20 \text{ cm}$ )

$k = 0.166$ , app. 2/3 of max.

$R = 1.2 \text{ m}$ ;  $H_B = 30 \text{ m}$ ;  $H_A = 3 \text{ m}$ ;

$Q_B = 37.8 \text{ tonnes}$  – buoyancy (payload)

$P_{sect} = 9.75 \text{ t}$ ;  $P_B = 97.5 \text{ t}$  – section, buoy weights

The ballast water column:  $H_w \approx 10 \text{ m}$ ,  $P_w = 31.6 \text{ t}$

### Ex. 2 (min. shell thickness $\Delta=12 \text{ cm}$ )

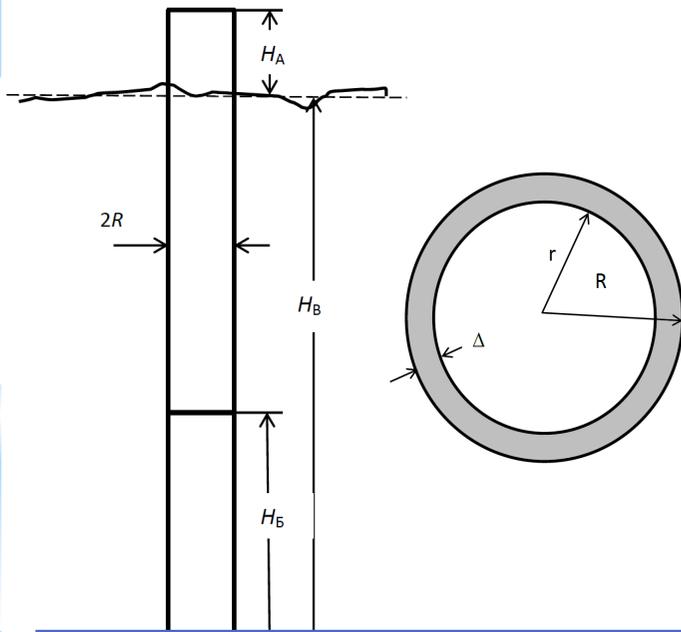
$k = 0.16$ , app. 2/3 of max.

$R = 0.75 \text{ m}$ ;  $H_B = 30 \text{ m}$ ;  $H_A = 3 \text{ m}$ ;

$Q_B = 15 \text{ tonnes}$  – buoyancy (payload)

$P_{sect} = 3.3 \text{ t}$ ;  $P_B = 38 \text{ t}$  – section, buoy weights

The ballast water column:  $H_w \approx 10 \text{ m}$ ,  $P_w = 12.3 \text{ t}$



**Solution:  $R > 0.55 \text{ m}$     Decision:  $R < 1.25 \text{ m}$**

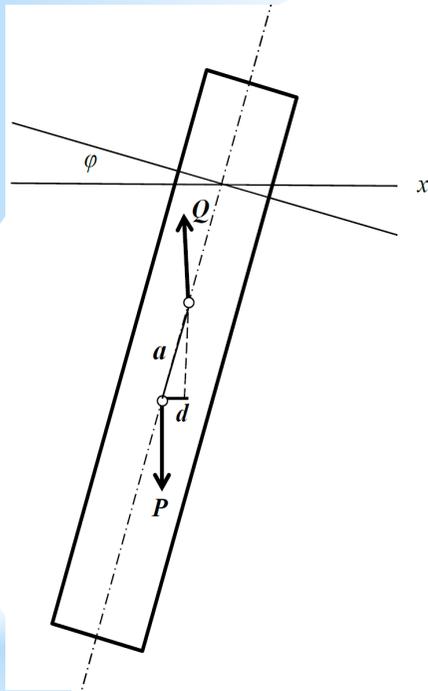
# Dynamics of the spar buoy

**Centers of loads and buoyancy:**  $z_g = \frac{\sum_i^n p_i z_{gi}}{\sum_i^n p_i}; \quad z_b = \frac{\sum_i^n q_i z_{bi}}{\sum_i^n q_i}$

**Righting moment:**  $M_B = Pa \sin \phi$

**Equations of motion:**  $(m_b + m_a)\ddot{z} + K_d \dot{z} + g\rho S_w z = 0$  – translation

$(J_b + J_a)\ddot{\phi} + k_d \dot{\phi} + gm_b a \phi = 0$  – rotation



$z$  - coordinate of the buoy center of loads;  $\phi$  - angle

$m_b, m_a$  - mass of the buoy, added liquid mass;

$K_d, k_d$  - damping rates,  $J_a, J_b$  - inertia moments;

$S_w$  - buoy cross-section,  $\rho$  - water density

## The buoy motion eigen frequencies

**( $R=1.2$  m,  $a=3$  m;  $m_a=136$  t,  $\Delta=15$  cm):**

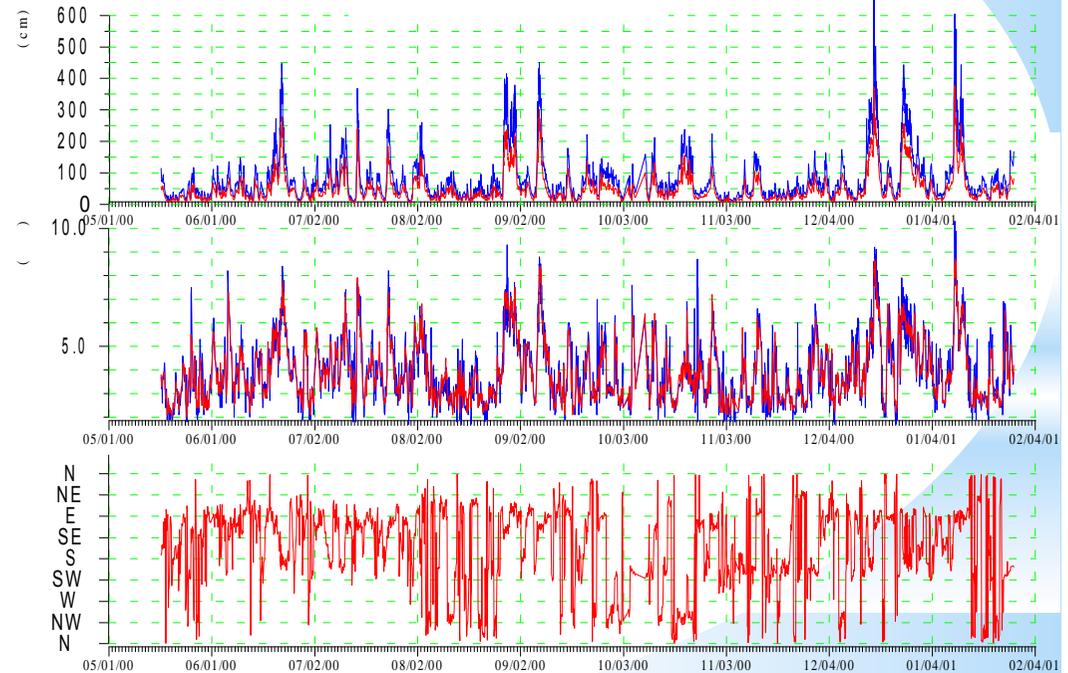
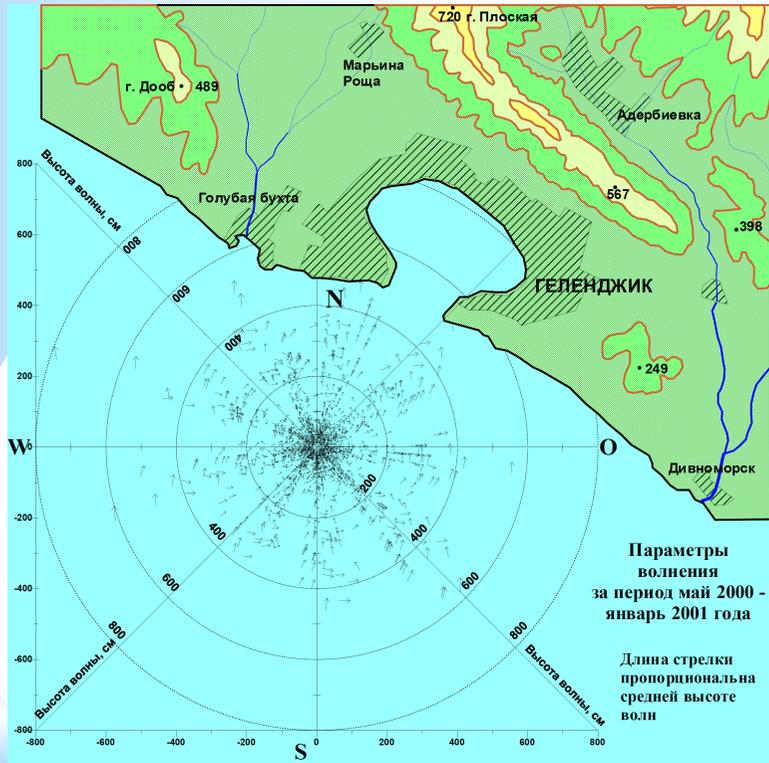
$f_{tr}=0.088$  Hz

$f_{rot}=0.051$  Hz

**The stabilization is reachable !**

**Solution: permanent ballast+adjusting water ballast**

# Monitoring of waves with Datawell buoy near the Gelendzik Bay (R. Kosyan, 1996 – 2003, NATO TU-WAVES Project)



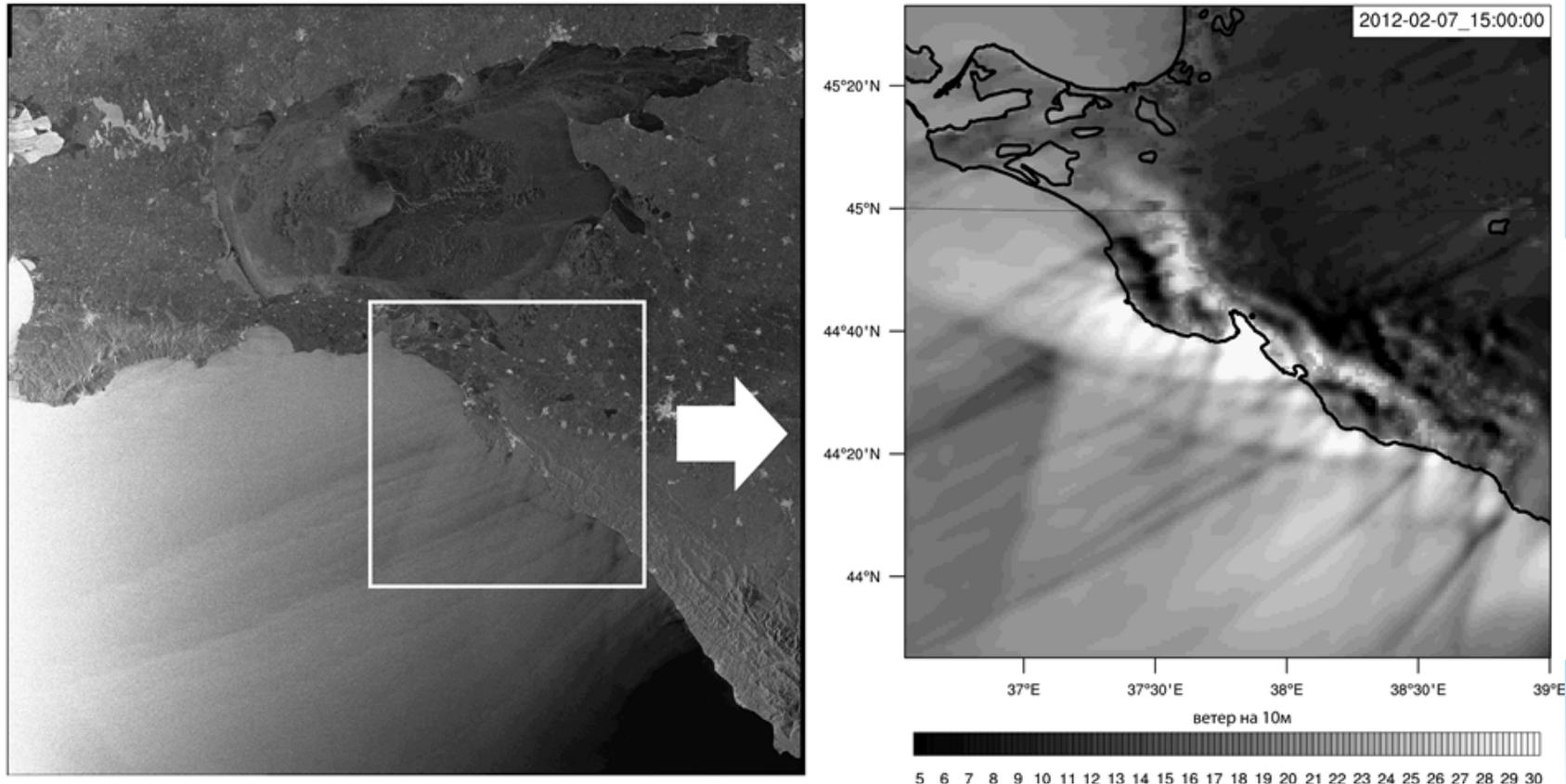
2000 - 2001

# Intermittency of wind field

during the Novorossiysk bora February, 7, 2012, 15.25 UTC

Left - Radarsat-1; Right - WRF-ARW model

A. Gavrikov & A. Ivanov, *Izv. Atm. and Ocean. Phys.*, 2015, V. 51, N. 5, pp. 546–556



How to estimate sea state in absence of reliable measurements of wind field?

# Wind-free method for the joint probability distribution of wave heights and periods (**Solution**)

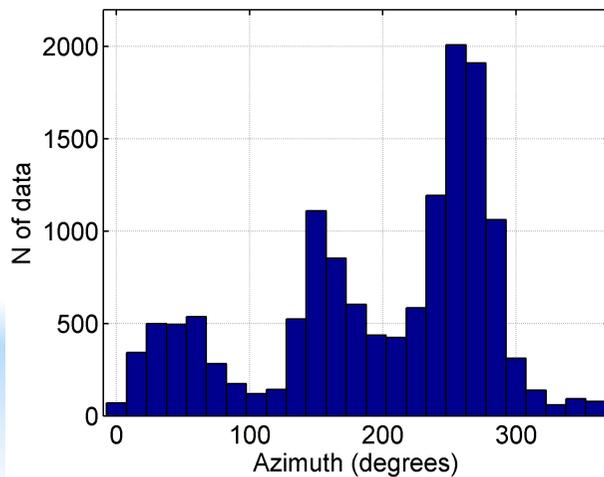


Wind-free invariant (Zakharov et al., JFM,2015)

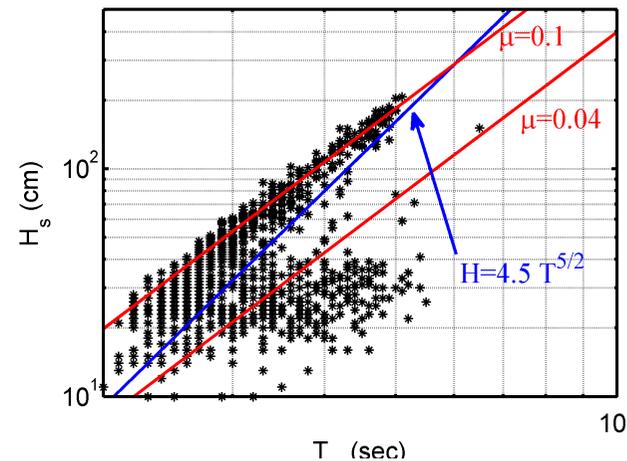
$$\mu^4 \nu = \alpha_0^3; \quad \alpha_0 \approx 0.62$$

$\mu$  – wave steepness;  $\nu = 2k_p x$  – number of waves

Sea state ( $H_s, T_p$ ) can be assessed for a given fetch without wind speed

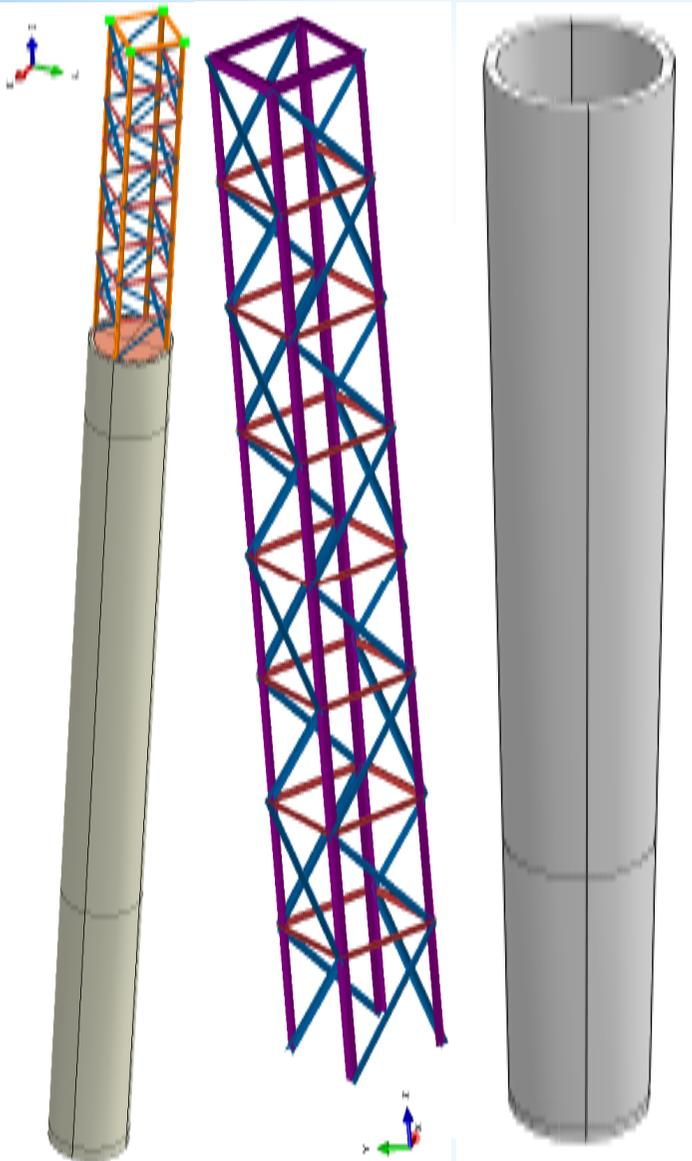


Number of measurements vs wave direction at the buoy site



Sea state estimate for offshore winds  
Wave measurements are between the wind-free invariant and a limiting wave steepness

# Strength of the buoy under effect of wind and waves



## Concrete part

Prestressed concrete tube 33 (30+3) m length

Diameter 2.4 meters (manufacturing + transportation)

Shell thickness 15 cm ( $> 12$  cm)

Bottom thickness 3 meters (permanent ballast)

Concrete C80 and higher (available everywhere)

## Steelwork

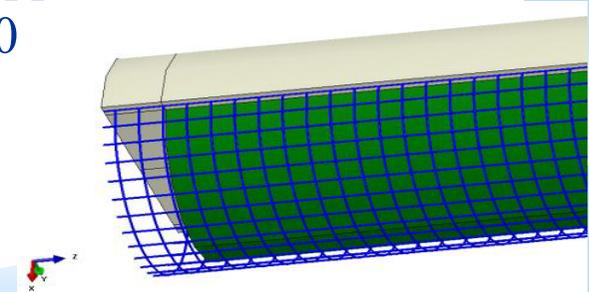
Steelwork height 12 meters

pointwise payloads: 4\*50kg at 15 m height,

4\*20 kg at 13.5 m, 4\*20 kg at 12 m

Heavy equipment in the upper section

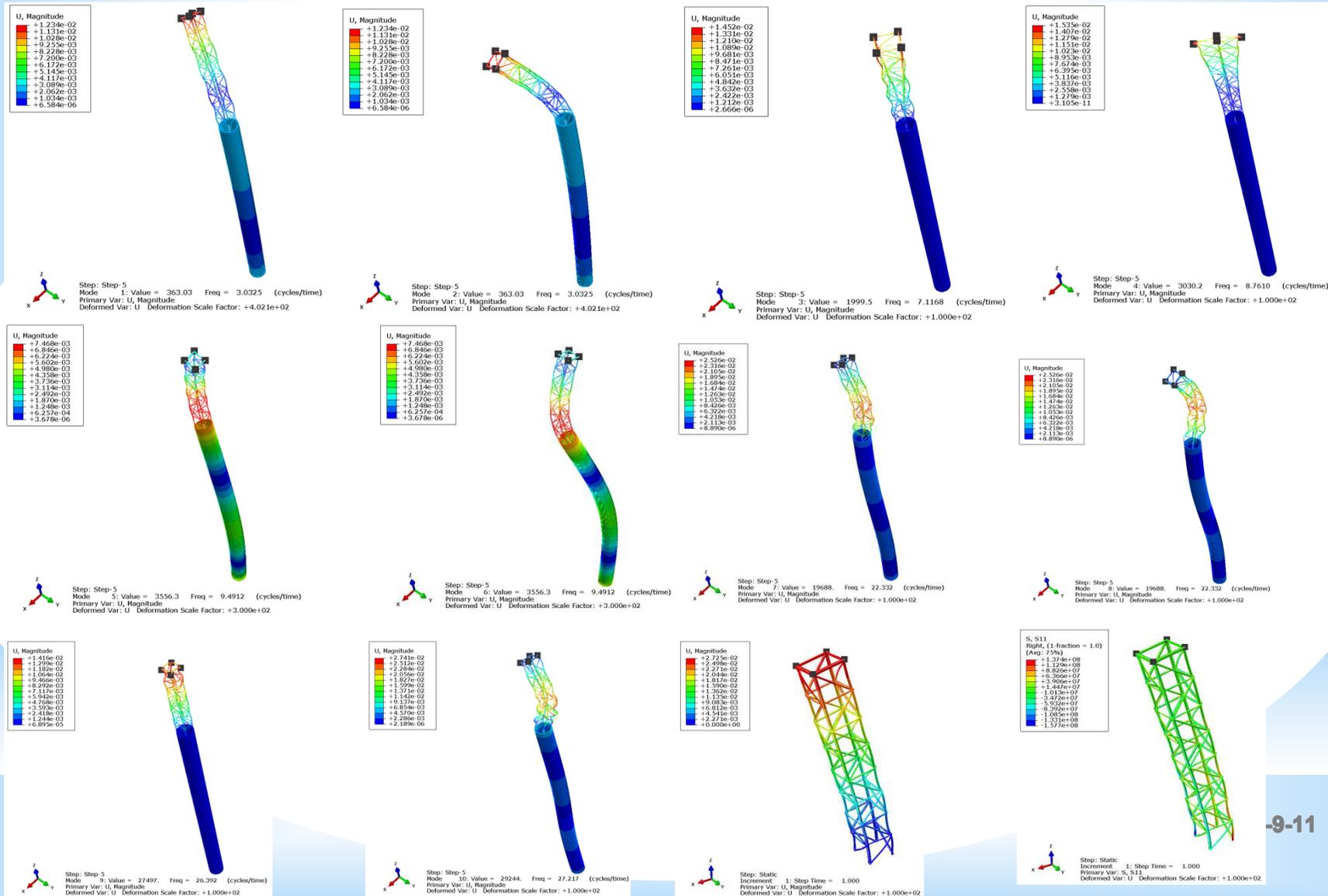
(batteries etc.) - totally 10



# Eigenmodes of the buoy deformation

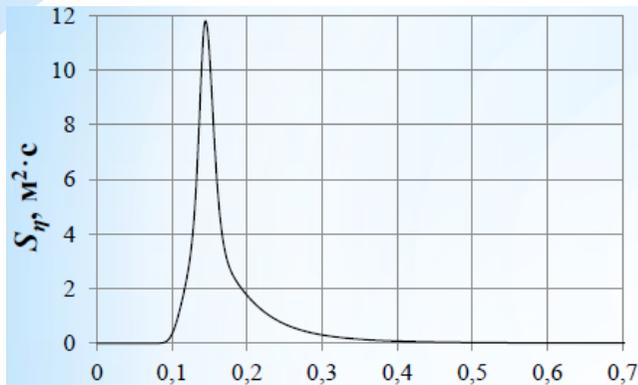
Frequencies are far from the wind wave range ( $>5\text{Hz}$ ) !!!

Loads are quasi-stationary

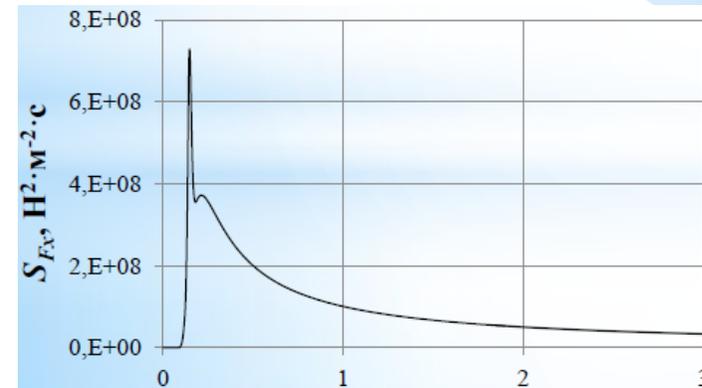


Wind and wave loads are alternating !!!  
 This is critical for the floating concrete structures !!!  
**Limiting load for compression is more than 15 times higher  
 than one for decompression (stretching)**

A JONSWAP-like parameterization of wave spectra for pairs ( $H_s$ ,  $T_p$ )



Spectral density of horizontal hydrodynamic loads



**Prestressed concrete structure is a regular (unavoidable) solution**

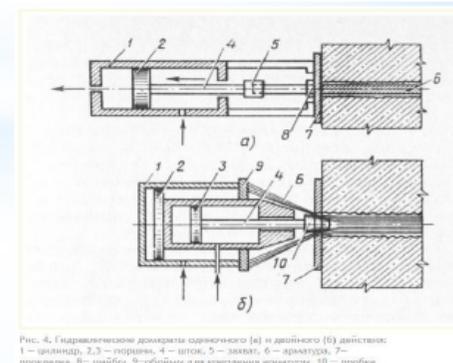


Рис. 4. Гидравлические домкраты одностороннего (а) и двустороннего (б) действия: 1 – цилиндр, 2,3 – поршни, 4 – шток, 5 – захват, 6 – арматура, 7 – приклад, 8 – шайбы, 9 – обмотка для крепления арматуры, 10 – пробка

# Regimes of alternating loads(Critical for the concrete structure)

## Solution: different time-scale scenarios

Event	Duration	H <sub>1/3</sub> , meters	T <sub>m</sub> , sec	N cycles	n of $\sigma$ for 99%
Max.storm-3	3 hrs	6.89	9.3	1 200	3.5
Max.storm-7	7hrs	6.00	9.06	2 800	3.9
Day	24 hrs	3.6	6.5	13 300	4.0
Weak	7 days	3.2/2.0	6.6/5.7	92 000	4.4
Month	30 days	2.6/1.4	6.0/4.7	430 000	4.9
Year	365 days	1.45	4.8	6 550 000	5.3
Decade	10 years	0.84	3.9	82 000 000	5.7

red - mean of daily (monthly) maxima

blue - maximal running mean for the whole duration

# Conditions of structural health

$\sigma_c > \sigma_t$  compression is stronger than decompression

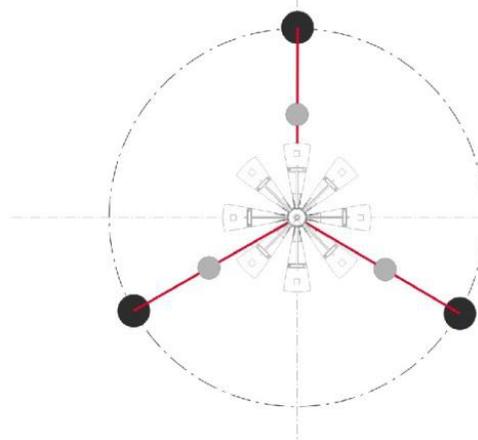
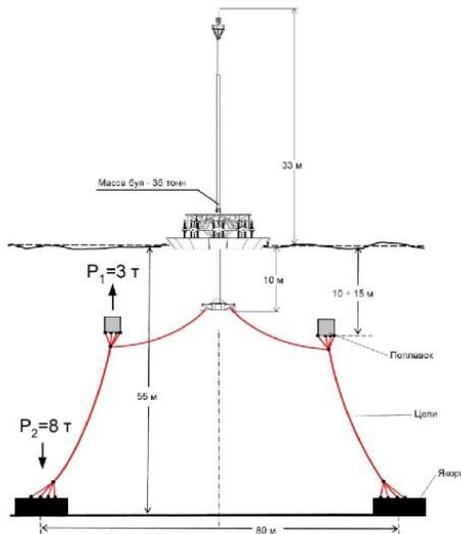
$\sigma_c + |\sigma_t| < R_b \Pi \gamma_i$  – all the stresses are weaker than the medium strength

	MS-3	MS-7	Day	Weak	Month	Year	Decade
$\Pi \gamma_i$	-	0.84	0.86	0.75	<b>0.67</b>	0.71	0.70

Regime **Month** is the most critical for assessment of the structural health

# Anchor system

Buoy is fixed by three anchors and three submerged floatages (the same as the buoy sections)



Ladies are for scaling only

Anchors can be made on-site



# Buoy deployment

The buoy is manufactured and transported to the place of deployment in the form of separate sections of about 3 m length. The buoy assembling is carried out on the shore. The stressed state of the structure is ensured by special jacks.



# Equipment

Navigation systems



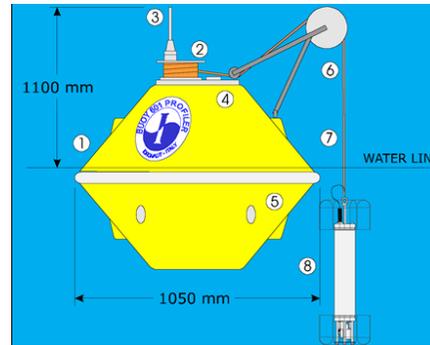
Structure control systems

Radars

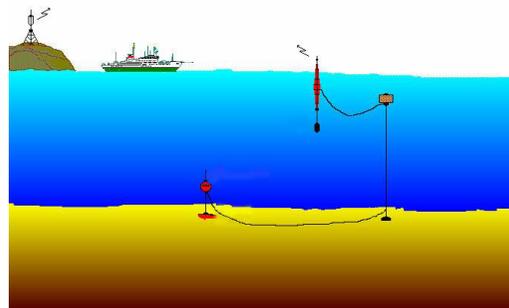


Telecommunication systems

Sea profilers



Seismic bottom stations



*etc.*

## Summary

- 1.The project of concrete stabilized spar buoy with the expected lifetime 10 years is presented for the Black Sea near-shore;
- 2.Strength and durability of the buoy are assessed. Specification of the concrete and metal constructions is formulated based on local wind sea conditions;
- 3.Specification of on-board instrumentation is sketched for monitoring the Black Sea near-shore.

*Thank you*

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